

Madras Municipal Commission
Report on the Present Conditions of
Madras Water Works

R
D.S. 211.111
N 27
15740

Madras Municipal Commission.
Report on the Present Conditions
of Madras water work

Book No : 15710

R
D 85-2114
N23

INT.

GINEER

MADRAS:

ALBERT MERCANTILE PRESS—S. BOWIE, PRINTER.

Since

Surveys when commenced

the Chief Engineer
prepare plans for
and which are
as stated in

Necessity of this
ment.

other methods

this I reply to

filter the water

4. The

Present open Channel

When designed it was supposed that the Channel would have a good current and the growth of vegetation be thus prevented. These expectations I regret to say have not been fulfilled, and at present it is most difficult to keep the Channel clear of weeds. This may partly be caused by the intermittent system of supply which we have been of late forced to adopt, and by which method the Water is as it were dammed up and allowed to rest in the Channel for a period of 6 hours during the day and 12 during the night. There is also another evil resulting from

a. Scheme

May last, having been relieved of a portion of
Madras, I have been able to give a great portion of
for acquiring the information considered necessary by
tion, and in addition to this, I have been able to
which is the source of the present supply,
Engineer for Irrigation
have referred to.

ners, firstly, why some
to Madras is needed at
scheme to be laid before you,
quality of water to Madras. In doing
reference as to whether it is preferable to
the Lake.

veyed to Madras from the Red Hills Lake by
open Channel which was calculated to deliver
gallons per head per day for a population of 545,000.

31.31, we are
 present arrangements, but
 can be raised against the
 a position that in drawing off the
 muddy mud.

9. It may be safely said that so
 At what Level potable Hills
 water can be had. means
 Madras can then only depend
 from 46 to 36—impure from
 arrangements) below that, i.e.
 cubic yards.

10. It cannot be stated with the
 Probabilities of future full again, or that
 supply. higher than a level
 had. Madras is therefore in the position
 potable water all the year round, in fact
 tank will
 during the year
 water can be
 certain supply of
 get such a belief

11. There are other objections to the
 Further objections to of its off-take. The Channel is eminently suited to be a breeding
 Channel. ing ground for fish and infusoria of all descriptions, and these
 eventually get into our pipes: did they not do so they would tend to keep the
 water clean, but as the pipes are not always full and as these infusoria attach

* When any levels are given the datum is Mean Sea Level.

three days. It is not
 in a great measure be prejudicial to the health
 15. Having thus shown, I hope satisfactorily, that the present supply is not
 to improve our present water supply, I now propose to consider schemes for its
 improvement.

16. One is not
 Schemes for Improvement
 means of supply can be
 The following are four ways in
 Channel and be filtered in

25.00 at the head, and
 the way to Madras, and
 to filter the water near Koo-
 above ground and then filter
 both pipes.
 Channel at its present level.

1. It is submitted with the remark
 that the filtrate of the last 5 feet in
 consequence of its being more costly.* Wm might of course arrange
 to have Engines permanently at the off-take of the Channel to raise the water when
 it fell below the level of 31.31, but as we must employ engines at Madras, this
 double set of machinery would be a most expensive arrangement.

* For reasons see page 7 para. 30.

23

and settling tanks could be adjacent to the north.

20. The level of this ground is about 17.00 above mean sea level. There are two methods of constructing the settling tanks and filter beds, the first being to have the settling tanks partly above ground (say at same level as the Chan) and the filter beds with their highest water being to have both filter and settling the loss of head below present arrangement. The second method the loss would be 22 feet but the loss would be very much greater than the latter. The second way would require less land.

21. The water pressure in the higher part of town is low. The present very

22. The water (taking the second method) settling and settling tanks and filters) would then have to be raised at least 22 feet to give the same pressure as at present, but it would be expedient to raise it at least 35 feet (this would be 42 feet above Mean Sea Level, and is 10 feet less of head than scheme proposed to be adopted.

* We must calculate loss of head at 1 foot above the bottom of the settling tank which is the same level as the water filters, as we cannot keep filling and emptying the settling tanks at the same time without desisting their utility.

ings at machine.

26. It will be observed that no allowance has been made for men as might be needed, or for fencing the compound. It is decidedly advisable I think from the proximity of the works to the town.

27. The proximity to the town is an objection as the water is very liable to be tainted by the foul atmosphere. The reservoir into which the water goes after filtration might be covered, but this would again add to the cost.

28. It will be advisable to detail it in a

	Rs.
Head Stance to Centre of Lake...	15,000
Price	3,000
Deeper than at present 197120	37,000
... ..	5,000
Replacing by syphons...	6,000
Under-pipe	8,100
Culverts	300
Removing and	5,50,000
Construction of compensating Reservoirs, &c.	1,68,750
Engines including buildings to raise water 28 feet.	4,000
Stand pipe and tower	2,19,450
27" Main from Koono masonry shaft 3 miles 4 furlongs and relaying present four half with same size of piping ...	4,000
Land	
Total...	10,20,600
Contingencies 10 per cent.	1,02,060
Rs...	11,22,660

great difference, in fact we might have very nearly approximated to the cost of this scheme by deducting half the cost of the alteration on the Channel, and adding the cost of this which comes to 2,19,450 for the piping and laying, which might be done below present Channel bed. The sides of the Channel might after being cut down level with the bottom, be made available as a road to the works, or it might be filled up for this purpose.

33. The cost of Maintenance in last scheme at per annum Rs. 646 per H. at work, on same rate for this, it would be Rs. 58,140 would not however be the same rate as there is 2 1/2 miles cartage done would probably cost Rs. 4,000 a year.

34. There is one great objection to both the is, that they Objections to Schemes Nos. 2 and 3. cannot well be executed with out the water from Madras for some considerable time. This objection could be got over (at of course increased cost) by a parallel Channel, the length being determinable on whether No. 1 or 2 scheme were adopted.

35. In making such a Channel it would be the least expensive way to make Constructing parallel Channel. it so, that one-half of the embankment was the bank of present Channel. Such a Channel from Red Hills to Madras would probably cost Rs. 1,50,000.* The cost of a parallel Channel to Koone

* I judge this from the cost of the present Channel 1,75,000. One half or 3 miles of Channel is in cutting, and the other in embankment, there would therefore be no saving in making a Channel 6 feet lower, far as the Earthwork is concerned on the first half, but there would be a saving by the earthwork and under Bridges on the second half.

PROPOSED SCHEME FOR FILTERING AND

ATTACHED TO THIS REPORT ARE THE FOLLOWING PLANS—

1. Small Scale plan showing routes to Red Hills and route adopted on sheet No. 1
2. Large Scale plan of route adopted, showing position of Tower, Pumping station, Filtering Beds, &c.... on sheet No. 2 & 8
3. Section of Route ... on do. No. 9 & 10
4. Section from off-take to Filters ... on do. No. 11 & 12
5. Plans of Tower and Elevation of Tower and Gangway ... on do. No. 13 & 15
6. Proper Elevation of Pumping Well, Engine and ... on do. No. 16
7. ... on do. No. 17 & 19
8. ... on do. No.

38. required to be determined was the position of the off-take. I have placed about 900 feet South West along the the Polal Sluice. Between this point and the the four parallel to the bund is practically level, and the slightly incline which might be had to the North East does not I think warrant the addition which would be incurred were the off-take placed North East of the Polal Sluice. Again to the South East the tank gets slightly shallower, so that we cannot move further in this direction; the point selected is I think decidedly the best place for the off-take.

39. The next point which demanded attention was the site for the tanks and filters. These I have placed on high ground as shewn, though not quite the highest ground. The loss of head however is very little, and indeed the very highest land is not of sufficient area to accommodate

5

4. The route to the end of the 42" Main at the Masonry Shaft, would be longer.

41. Having thus fixed the ends of the routes, the alternative routes were next considered as shown in sheet No. 7.

42. I decided on the route marked No. 1 through Perambore for the following reasons.

43. The route by the road is too close to the railway, and that is a disadvantage. The other routes are shorter than No. 1, but they pass away from the railway.

44. The route marked No. 1, is altogether the best. It is the nearest to the railway, and I think this is a great desideratum. It passes near the Perambore Works of the Madras Railway, where there are a number of people who will gladly use the water, and the Company itself. I think it will be glad to take advantage of it. The route avoids all tanks through which piping could not be laid in the wet season. It avoids passing near houses or through any property which might be injured in laying the piping, and it, as far as possible, avoids topes through which it is costly to lay piping, and which might probably injure the pipes in future or render it difficult to get at them in case it were required to do so. It was impossible without increasing the length of the piping considerably to avoid these topes altogether. The route also passes near the Lunatic Asylum which may also be provided with water.

File
done

50. In the first place we should require a lake; if raised high enough we might reduce the size and cost of the Main to Madras below what it is in the scheme I propose.† But this advantage is more than counterbalanced by this, that unless at Madras we have a compensating reservoir at a high elevation, we must have engines of double the power we would otherwise need, and still a compensating reservoir at a low level from which they would pump, so as to allow of filtration proceeding regularly, and we must also have engines at the Red Hills—and that of very little less power than the only engines in the scheme I am proposing. Indeed there is a great deal of construction will be more, if we have the water filtered near Madras, and as the ground is on a very high level, this is below many parts of the town.

WATER TOWER.

51. This I propose to place at the point previously described and at a distance of 74 ft. from the top of the inner slope of the bund. It is not quite at the lowest level of the bed of the tank, but a small Channel will be constructed as shown, so that water can be obtained down to the full depth required. The lowest level from which I propose to draw off is 22.00. The tower I propose to found at a level of 14.00 on concrete made of Morgan's cement which could be lowered through water if necessary, should a difficulty be

* See note by Secretary to Government Department Public Works Irrigation.

† It would not require to be raised quite to the level of the proposed filter beds, to do this as the size of the Main in the scheme proposed is increased on account of the water not being drawn regularly all day, and in this alternative proposal, the compensatory reservoir would be in Madras.

top of the girders will be cased in wood to form a hand-railing, is divided into two spans each of 33 feet 1 end of tower

shewn, and the opposite masonry will support sheet

CULVERT.

52. I have shewn a culvert for conveying the pumping main from the tower through the embankment, and this has an entrance placed as shewn beyond the outer face of the bund.

53. I have shewn this in deference to the views of the Chief Engineer for Irrigation, but I beg to submit for consideration whether such is needed, and whether a large amount of pipe and two or three collars of brickwork at intervals is necessary to prevent any flow of water from the tank through the bund. The present wall as far as I am aware, and this may be a reason against an alternative proposal, I however record such for consideration.

54. If the culvert be constructed, the piping will be supported on cross walls each nine feet apart as shewn.

55. The entrance to the culvert will be covered over, and a trap door placed in the cover with a ladder underneath for admission.

GANGWAY.

56. The Gangway consists of two lattice girders, their top and bottom formed of T irons, and their bracings of flat iron supported by

we can p. I have made some remark as I do not con.

Foundations unsafe at back of bund. immediately behind the tank probability be very soft and after

I am led to believe that there is a leakage through from the tank under bund. The foundations for the Engines and pumps at this site would be expensive, and as the ground is also liable to flooding, they would require to be raised considerably above its level. There is again the risk that the operations in excavation might tend to create a rush of water through the bund and the continual vibration of large pumping Engines would not I think be favorable to its safety. On the plan I have marked two sites, Nos. 1 and 2 where I think they would be favorably placed.

59. I am not sufficiently acquainted with the construction of the force pumps used in water works to state the following positively, but I believe that at whatever level they are placed they will require to pump from that level whether the tank be at a higher level or not; with however an atmospheric pump, we can always draw safely 20 feet, and on this account I prefer site No. 2 which is at a higher level, because we would be able with an atmospheric and force pump to take the full advantage of the level of water in the Lake and only have to draw from that level. Thus suppose the inlet valve of the pump were at a level of 42.00, if the tank were at 42.00 we should only have to force from 42.00 up to the level of the filters and at most only have to draw between 42.00 and at whatever level the tank stood. If this same advantage is to be gained by a force pump, then there

in site No. 2, except perhaps that is on more accessible ground, able to floods as site No. 1. There is, however, an objection and it is this that we must draw atmospherically through s could originally be in so, or what it It would hjec- d

61. The level of 70 above sea level can be raised to a level of 20 feet to a level of 70 above sea level. 100,000 gallons per day.

62. The Power required for this is found by the following calculation.

$$\begin{array}{l} \text{Calculations for Engines.} \\ \frac{\text{gallons. lbs.}}{\text{hrs. min.}} = \frac{6,000,000 \times 10 \times 50 \text{ feet}}{33,000 \times 24 \times 60} = \text{say 64 H. P.} \end{array}$$

this, however, is only the nominal H. P. required. It is necessary to have reserve power in case of accident, and I therefore propose three Engines, any two of which can do the work, so that we require three Engines each 32 H. P. nominal.

63. For Engines of this H. P. (nominal) about 1/4 of area for each H. P. is allowed by the makers, and the steam at 30 lbs. pressure over six inches, and the speed 200 feet per minute, we have the actual H. P., for which amount work, viz.:-

$$\frac{\text{H. P. inch. lbs. feet}}{33,000} = \frac{32 \times 7 \times 30 \times 200}{33,000} = 41^* \text{ H. P. actual.}$$

This is the amount for which I have allowed in the estimate (3 Engines of 41 H. P. each.) We have already seen that the area of the cylinders is 224" therefore the diameter = $\sqrt{\frac{224}{7854}} = 17$ inches.

Stroke of Engine.

64. The stroke is usually twice the diameter this gives 34" = 2.83 feet.

This brings us to the calculations for the size of the pumps.

* It is supposed that this amount will also cover the extra head due to friction in the pumping main.

PUMPS.

65. Let there be 3 pumps

No. of pumps.

same

main. The foundations leads to economy. A similar plan has been adopted at the Eastbourne Water Works with success. The air vessels should be of about 38 cubic feet in capacity.

67. The cost of pumping Engines varies very considerably. The cost of the Engines at Cork Water Works (Sir J. BENSON) was £55 per H. P. and at the London Sewage Works the Engines were estimated at £70 per H. P. Both the above included Engine and Boiler houses, Chimneys, &c. The Board of Health have adopted a standard of £50 per H. P. and Mr. NEVILLE in his work on Hydraulics says that the cost per H. P. will vary from £50 to £100. I have taken the extreme limit and put the cost per H. P. nominal at Rupees 1,200 allowing for contingencies and carriage, exchange, &c. this amount will I think cover all probable expenses, and I include the Engine House and Buildings in this sum.

PUMPING MAIN.

68. I propose that the pumping Main be 18" in diameter. The amount of water raised per minute by the Engines is 666 cubic feet and this would equal a velocity through one pipe of 6.0 feet per second which is not excessive. This main should be furnished with two or three intermediate stop valves to prevent the water returning in case of accident and a loaded blow off valve will also be necessary for each pump.

FILTER BEDS AND SETTLING TANKS.

69. These are fully detailed in the drawings attached to this report, see sheets No. 7 and 8. In designing them, economy was studied. By the plan the water on ground reservoir not over

in 5,000

will be

being

therefore

available

able to com.

cubic feet

gals.

$$45,000 \times 8 \times 6 = 2,160,000 = 13,500,000.$$

Time of filling and emptying tanks.

As however one tank will always be out of use for the purpose of cleaning, we can only calculate on 11,250,000 gallons; as each tank holds 2½ million gallons, it will take 9 hours to empty itself—so that it will take 45 hours to empty the five tanks as however it will also take 9 hours to fill each of them, we must deduct this amount and thus the water will be allowed 36 hours for settling, when only five tanks are in use; but 45 hours when all the six are in use. This time I think will be sufficient for whatever state in which the Red Hills Tank may be. When the tanks require to be cleaned, the remaining water in them will be let off by a drain which is sunk below the level of the tank, and towards which the beds of the tanks slope. The drain is on a gradient of 1 in 500. The beds of the tanks being a smooth surface, they will be readily cleaned, first by washing them with remaining water 12" deep, and by then attaching a hose to the supply pipe of the tank. These drains will be led into tanks which are around the base of the hill—the water may therefore be made available for irrigation or other purposes to a small extent. These drains will be partly tile drains, and partly open cuts or Channels.

FILTER BEDS.

71. There are five filter beds, one of which will always be out of use for cleaning purposes. They have each an area of 2,336 square yards, or with 4 filters 9,344 square yards. The rate of filtration will be 642 gals. per square yard in 24 hours—which this gives 6,000,000 which is

the quantity required. This would be about 5.5 inches per hour. The rate of

Rate of Thames Com- filtration to be efficient should never exceed 6 inches per hour. pany.

This rate has however sometimes been exceeded, and the following table exhibits the results. The Thames Com- filter beds.

73.

Distribution how effect.

The well, pipes lead to enter the filter bed at a slightly higher level. The valves are placed at the various collecting wells.

74.

Filtering Media.

media, which are—

- 2 feet fine sand.
- 1 foot sand size of shot.
- 1 foot gravel size of walnuts.
- 1 foot gravel large size.*

75. The lower layer are the drains 6" and 9" in breadth according to their position in the filter bed. I propose that they be of 4½" brick walls and flat tiles laid over them. The bricks will be perforated as also the horizontal tiles—these it is probable can be made in this country.

76. There are three available methods of forming the sides and floors of the settling tanks and filter beds. They are—1st of Brickwork laid in Chumara and pointed with Cement, and 2nd Laterite similar to that used in the Red Hills Tank Bund and plastered with Morgan's or Portland cement. I have estimated for executing the works in either way—in either case I propose to lay the masonry on puddle one foot thick.

*The supply of filtering material is an important point. Sand can be had best I fancy from the Cortilliar River. Small gravel may be got at the Red Hills, their upper strata are composed of it. It would however require extensive washing. Larger gravel might probably be had from the Cortilliar, but there can be little difficulty in procuring this. The lower layers of the filter beds seldom if ever acquire renewal.

77. After filtration the water is carried by drains and pipes into the main collecting well—the pipes from Nos. 1 and 2 filter will be carried under filter No. 3. and the pipe brought to the same at the well as the other filters. From this collecting well the water

When

filter in

valve

lves

able

the

ill

rs

ater

water in the tank would not be allowed to draw off in 6 hours, but as the filters only pass 1,500,000 gals. in this time, we require to make up for this difference of rate in receipt and supply. It is also necessary to keep always a head of water above the pipe. I have allowed for a reservoir able to contain 2,000,000 gals. if necessary, but except in exceptional circumstances, all this quantity would not be needed. The following shows approximately the varying rate at which water is taken in Madras.

		Gallons.			
		A.M. A.M.	Let	1,500,000 be the amount in tank at 6 A. M.	
3 hours	Filtered from 6 to 9	...	750,000		
	Drawn from 6 to 9	...	2,250,000		
6 hours	Filtered 9 to 3	A.M. P.M.	1,500,000	This will be the amount in tank 9 A.M.	
	Drawn 9 to 3	P.M. P.M.	2,250,000		
3 hours	Filtered 3 to 6	...	1,250,000	do.	do. at 3 P.M.
	Drawn 3 to 6	...	1,750,000		
12 hours	Filtered 6 to 6	P.M. A.M.	2,000,000	do.	do. at 6 P.M.
	Drawn 6 to 6	...	500,000		
Total 24 hours			3,000,000		
			3,500,000		
			2,000,000		
			1,500,000	do.	do. at 6 A.M.

It will be observed that half the quantity is drawn 6 hours, viz., from 6 to 9 A. M. and 3 to 6 P. M.

The lowest at which the tank stands is 500,000 gallons and the level of the water when the tank holds 2,000,000 gallons is 10 feet, there will be less than 2½ feet in the tank, or perhaps rather more (say 3 feet) in the tank sloping sides. As designed, the settling tanks above the main collecting well when the water level is low

they are

81. The

It is a large box rounded of brickwork. The upper floor is formed of pebbles. The sand to be cleaned is laid; between the floors the water is admitted at some pressure from the pumping Main. It enters between the floors and passing up through the sand which is being vigorously stirred by men with peculiar spades, carries off any earthy substance with it. The box is divided into three tubs, so that a large or small quantity can be washed. Two of these boxes are shewn on the general plan of the filter beds, &c. and have attached sheds for storing sand cleaned and that not so.

PIPES, VALVES, &c.

82. On the pumping main reaching the main valve house, it is divided into three branches to the three pairs of tanks, and in this house are the valves for cutting off the supply of water from any pair of tanks. On approaching the settling tanks, each branch is again divided into two, with valves to each tank. From the valve house just referred to, there will be an electric communication with the Engine House regulating the supply from the pumps to the settling tanks, or a self-acting communication could be made by allowing the water when it gets high enough in the tanks to run back through a small pipe to the Engine House and blow a whistle. This valve house should be erected with an upper story so as to allow of the levels of the water in the filter beds and tanks being carefully watched. The next most important valves are these at the main collecting well, (see drawing No. sheet N^o 18), there will also be a valve on the Main from the Reservoir to the Town and valves will also be placed on the other pipes where necessary.

MAIN FROM RESERVOIR TO MADRAS.

83. The size of the Main depends on the quantity to be supplied, the rate at which it must be supplied and the head of water obtainable.—
 The quantity to be supplied is 6,000,000 gallons per day, the demand will exceed this, or of the pipe for the day—

at a level of 50 and this gives 52 feet towards the town is about the level; at some points the branches are higher. This will not however to a great extent influence the calculation for the main.

84. I have calculated the diameter of the main by the two following formula which give the results set against them—

- inches.
- * Beardmore's which gives the diameter as 34.40
 - † Hawkesley's do do as 37.33

By NEVILLE's tables the velocity of discharge from a 36 inch pipe under same conditions is 38.8 inches per second, and this would give 22.2 cubic feet per minute which is the amount we require.

85. In Calcutta the size of the pipes was determined by Hawkesley's formula and calculated to supply 6,150,000 gallons per diem at an average rate, (the compensating reservoir is in this case at end of the main next Calcutta), but by experiments made in 1870, it was found that the actual discharge was at least 8 million gallons an increase of 30 per cent. I think I am therefore warranted in estimating for a 36 inch main and this is what I have done.

86. The present main (42") is of course larger than this, because it has less head (23 feet less); but as this head is obtained only half a mile distant from the point of junction, the pipe is on that account

* Beardmore's formula is $D = .235 \sqrt{\frac{l \times q}{h}}$ in which, D = diameter in feet, l and h the length of pipes and head in feet and q the quantity to be discharged in cubic feet per second.

† Hawkesley's formula is $D = \frac{1}{15} \sqrt{\frac{q^2 l}{h}}$ in which q = No. of gallons to be discharged per hour, l the length of pipe in yards, and h the height in feet and D the diameter of the pipe in inches.

less in diameter than it would be were this head obtained at Red Hills or a similar distance away.

87. I think it will be expedient to replace the part of this main between the point of junction of masonry pipes suited to the consumption of the junction of the pipes.

an
 here, so
 between
 cannot be
 way to Madras
 not at present

88. The main which would close on any break taking great increase in the velocity of flow.

be needed, and a few stop valves should be provided to divide the main into short districts to enable it to be readily repaired if need be.

89. I have now described the works which I propose, I think fully enough to shew their main points, and to enable it to be said whether they can be executed for the amount set down in the estimate.

90. There is however another point to be referred to, and that is the construction of a storage reservoir in the old Lunatic Asylum compound, the cost of which I have not included in the scheme. The level of the ground is about 12.00 and its area is about 11½ acres. The highest points which we need to supply are at a level of 20.00. It is therefore evident that any reservoir must be above the level of the ground. I propose that if it is thought absolutely necessary to have a storage reservoir, to construct it so that its highest water level may be 10 feet above ground level, or a level of 22.00; if the reservoir were made of an area equal to about 384,000 square feet for which there is an ample space, the reservoir would hold four days supply, and this is a much greater time than the main if properly laid would ever take to repair. Two days might be taken as the extent I think, and if so, the reservoir would only fall 5 feet and be at a level of 17.00 which would still supply Black Town and other low parts of the town* in which are the greater parts of the population. The cost of such a reservoir of masonry may be approximately stated at Rs. 2,00,000, and if covered as it should be, it would cost considerably more. It would be unsafe I think to put a earthen revetted embankment so near the town.

* If this plan is adopted the pipes over the Elephant Gate and Hospital Bridges must be relaid under the Canal instead of over the Bridge as they will be too high when this reservoir falls.

91. I think it necessary to institute a comparison between the areas of the works allowed in this scheme for settling and filtration—and those at Calcutta and it is as follows :—

Comparison between this scheme and Calcutta.

Scheme for Madras Calcutta.

Settling Tanks
Filter Beds

feet.

s, that

the

that

in

ing

proper

6 million

r cleaning

looking for the very different quality

of this scheme. I have already shewn

that the area here is more sufficient, and Calcutta can be no guide in this respect.

93. To give however other evidence. Take the five "Thames Companies" viz., The West Middlesex ; Grand Junction ; Southwark and Vauxhall ; Lambeth and Chelsea. The average area allowed by these Companies for subsiding reservoirs is 4.22 square yards and for filters 2.34 square yards per 1000 gallons in 24 hours, this for the scheme under consideration would be

	Sq. yds.	Sq. feet.
Settling Tanks $\frac{6,000,000 \times 4.22}{1,000}$	= 25,320	= 227,880
Filter Beds $\frac{6,000,000 \times 2.34}{1,000}$	= 14,040	= 120,360

94. It will be seen that the Calcutta area is greatly in excess in both cases. The settling tanks I propose are slightly larger in proportion while the filter beds are slightly less—it will, however, I think be admitted that Thames Water is never so clear as water from the Red Hills Lake taken from near the surface.

ESTIMATE.

95. In making the estimate I have not gone fully into detail, only sufficiently so to get at a close approximation to the cost. I have allowed ample rates, and having been guided by prices lately paid in the district.

* The depths in this scheme and at Calcutta are the same.

† See Jackson on Calcutta Water Works.

96. The following is my Estimate of the works, such as I have detailed in the foregoing reports and on the drawings attached :—

PARTICULARS.	If of Brickwork.		If of Laterite.	
	Rs.	P.	Rs.	P.
Towers				
Valve House				
Offices, Stores, &c.				
Gateway				
Sand washing Boxes, (2) &c., including Piping				
Drains, Pipe and Tile, &c.				
6.28 miles of 36" Main to Madras, including laying and carting an average of 6 miles				
Stop, Self-acting, and Air Valves, on do.				
Valves, Piping, Branches, Junctions, &c., and Machinery at Filters				
Total...	12,42,122	0 0	9,64,072	0 0
Add for contingencies, &c., 10 per cent...	1,24,212	0 0	96,408	0 0
Grand Total...	13,66,334	0 0	10,60,480	0 0

The above estimate shews the cost of the several works in Brickwork and in Laterite. The tower which I have shewn to be of Brickwork in each estimate might perhaps be also of laterite, but I fear that the latter material is too porous to make satisfactory work in the tower. The difference of the cost of Laterite and Brickwork is very great. I have estimated that good Brickwork can be done at Rs. 12 per cubic yard, which is a moderate allowance. Laterite Pitching which I propose should be used in settling tanks and filter is said to have cost at Red Hills Tank Annas 8 per cubic yard. I am unable to see how it can have been done for this amount. The price I have estimated these works at is Rs. 4 per cubic yard, and this includes plastering with MORGAN'S cement;—and I think there is little doubt that the work can be executed for this amount. It may then be safely considered that the cost of these works will not exceed 10½ Lacs—were they to be carried out in full.

97. The consumption in Madras at present however is only 7½ gallons per head instead of 15 gallons for which these works are designed. Therefore the full Engine power would not be at first required. As the three Engines estimated for can do one and a half times the work, one Engine will do half the work, and if two Engines are ordered at present we have one for the contingency of the other breaking down. We may then deduct the cost of one

Engine less the cost of Buildings, say this is Rupees 1,000 per H. P. nominal and
H. P. Rs. Rs.
as decreases the estimate by $32 \times 1,000 = 32,000$.

98. Again the filter beds will be more than sufficient, and we may do without
these and one pair of tanks, would be about Rs. 1,15,000, so that
the amount first expended would not be more than

Maintenance

the rates at which I have estimated

are shewn whether the estimate

various

be

which

the

rial

any's Wh

asked to

for some cost.

I also to

in the cost of cartage

uld result.

the Red

MAINTENANCE.

101. The cost of Maintenance will depend greatly on the cost of coal which
I have put down at Rs. 25 per ton delivered at the Red Hills. The monthly cost
would probably be as follows:—

	Rs.
1 Superintendent at Rs. 250	250
3 Drivers at Rs. 60	180
3 Firemen at Rs. 20	60
eq. 112 Tons Coal ($4\frac{1}{2}$ lbs. per actual H. P. (82) per hour at work	2,800
12 Coolies at Rs. 6	72
10 Turncocks Rs. 20	200
Sand for filters and cleaning*	550
Oil contingencies, &c.	300
Total...	4,412

This is equal to Rs. 53,044 per annum. As before shewn the present consumption
is only $7\frac{1}{2}$ gallons instead of 15 gallons per head per diem, and the above is
for pumping the whole 15 gallons. The cost for the $7\frac{1}{2}$ gallons per head per diem
would amount to not more than 30,000 or 36,000 per annum, and this latter
amount capitalized at 5 per cent. = Rs. 720,000. If we add this amount to the
estimate of 9,00,000 it gives us Rupees 9,720,000 which will be the first total cost.

102. In concluding this report, I wish to remark that it includes with the
accompanying plans, sections, &c., all the information desired by Government. If
the estimate be adopted and the design for the Tower Culvert &c., be approved of by
the Chief Engineer for Irrigation these works may be started at once as pointed

* The cleaning of filters has been taken at Rs. 2-8 per million gallons filtered in 24 hours. In England it is
about 10s. or 5 Rs.

out by Colonel MULLINS in G. O. under reference. Probably every day now lost
will increase their cost.

103. I cannot venture to hope that Government will accept the whole scheme
as proposed by me without further professional advice, but I venture to ask, unless
some practical objections are received against my proposals that they be
submitted on any doubtful consideration some consulting engineer of eminence
may be necessary.

104. No

at increase

capacity to

105. There is no

capacity departmentally as

cost, and is much less time and much more

contractor, and of this I have had some experience

respectfully to ask that His Grace the Governor

views as to the placing of the works in the hands of a

recommend

contractor

106. I beg that any omissions in this report, though they may be criticized
as leniently as possible. I have had little or no assistance in their preparation,
and it was necessary for me on account of other duties to spend as short a time
on this work as possible.

I have the honor to be,

Sir,

Your most obedient Servant,

J. A. JONES, Assoc. INST. C. E.

Executive Engineer,

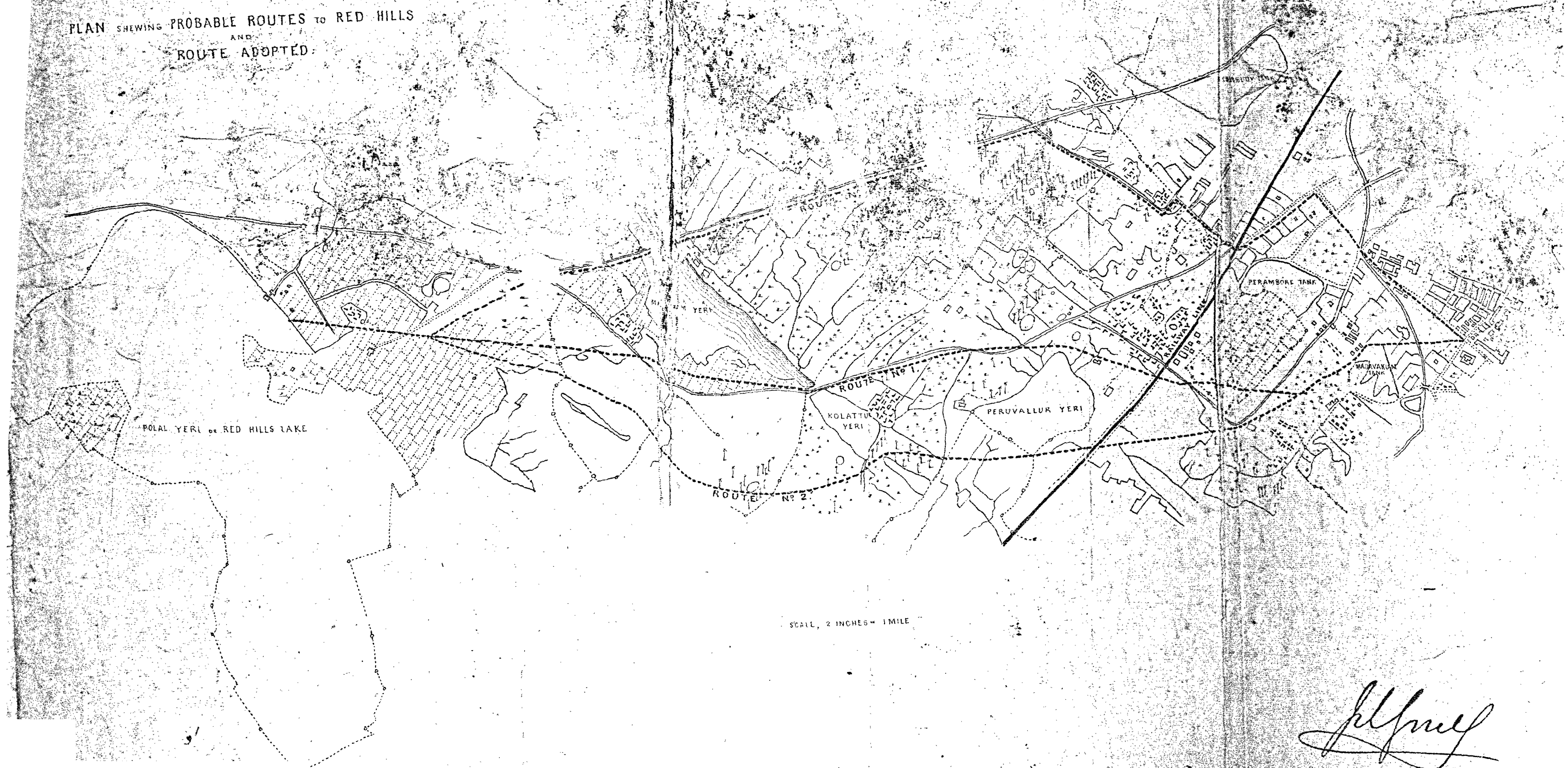
Madras Municipal Commission.

Madras

June 1877.



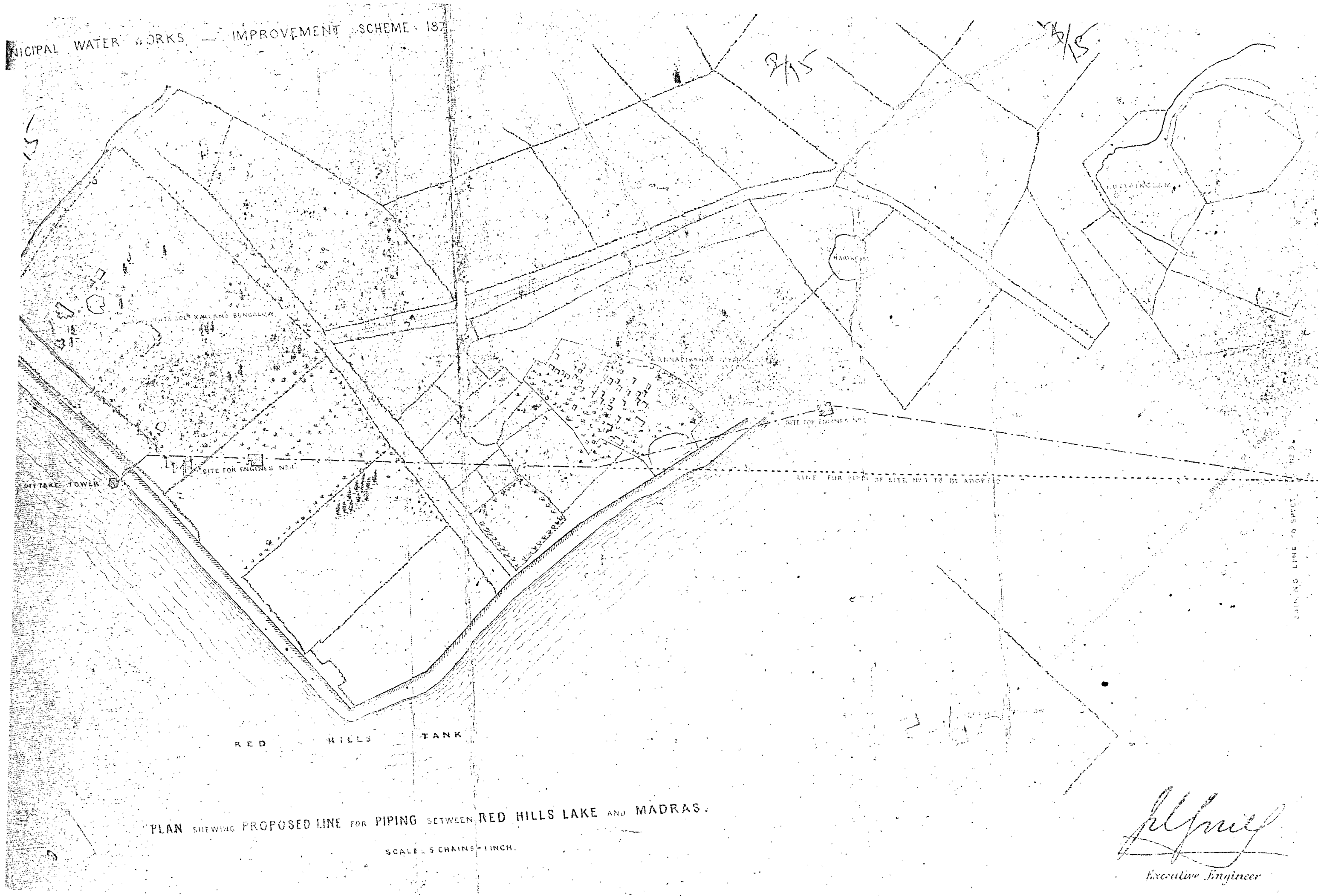
PLAN SHEWING PROBABLE ROUTES TO RED HILLS
AND
ROUTE ADOPTED.



SCALE, 2 INCHES = 1 MILE

J. L. S. S.
Executive Engineer

MUNICIPAL WATER WORKS — IMPROVEMENT SCHEME, 1877



3/16

2/16

NAYARUR KOLAM

KATTARIVEDU

[Signature]

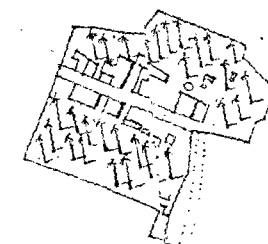
Executive Engineer.

MUNICIPAL WATER WORKS — IMPROVEMENT SCHEME 1877.

2/17 3/17

MADAVARAM ARI

KALPALAIYAM



JOINING LINE TO SHEET No 5

Handwritten signature



34

APPENDIX No. I.

Table Shewing rates used to form Estimate.

Nos.	Particulars		RATES.		
			Rs.	A.	P.
1	Brickwork per	Cubic yard.	12	0	0
2	Laterite built in Chunam and plastered with } Morgan's cement... .. }	"	4	0	0
3	Clay Puddling	"	0	8	0
4	36" Piping delivered in Madras	Ton	80	0	0
5	Laying do per	Lineal yard.	3	8	0
6	18" Piping delivered in Madras	Ton	80	0	0
7	Laying do	Lineal yard.	2	12	0
8	Cartage of Piping per	Ton mile.	0	3	0
9	Excavation at site of Filters &c.	Cubic yard.	0	4	0
10	Cost of Valves, Bends, Sluices, &c. taken at same rates as Madras W. W. contract.				

25

APPENDIX II.

The following Statement shews the cost of "Gravitation and Pumping" works constructed in England for a supply of 20 gallons per head per day.—

Town.	Population.	Maximum daily supply.	Total Cost.	Cost for a supply of 20 gallons per head per diem.
		Gallons.	£	£
Gosport ...	22,000	222,000	21,000	1.75
Hartlepool ...	30,000	29,00,000	75,000	.75
Kendal ...	13,000	300,000	15,500	1.03
Liverpool ...	6,20,000	16,500,000	2,000,000	2.42
The Potteries ...	1,30,000	3,000,000	215,157	1.43
Merthyr ...	55,000	1,500,000	84,000	1.12
Newport ...	30,000	15,000,000	52,000	0.07

The cost of the scheme now proposed for Madras is, say Rs. 10,50,000 say	...	£1,05,000
It has cost already	Rs. 12,80,000	£1,28,000
	Total...	£2,33,000

This amount for a population of 400,000 = 0.58 for 15 gallons per head or say for 20 gallons ... 0.77

The average of the above towns is ... 1.37 for 20 gallons per head,

The cost of Calcutta Works was
 £570,596 for 429,535 people = per head... £ 1.35 for 15 gallons nearly
 (say) 1.77 for 20 gallons per head.

It will be noted therefore that Madras even after this scheme has been carried out will be supplied with water at almost half the rate of the English towns noted and at considerably less than half what the Calcutta works cost.

J. A. J.

26

APPENDIX III.

MEMORANDUM on a method of supplying Madras direct through a pipe without pumping, and by which the water may be able to be drawn when the Lake stands at any level—

- I.—The bottom of Red Hills Lake at lowest point is 20'00.
- II.—The fall between this level and the point at which the main is joined in the scheme to which this memorandum is attached is 20 feet.
- III.—If a continuous pipe be laid to deliver water while the Lake is at any level not lower than 22'00, no part of the pipe can be above this level, and none of it should be higher than the line of vertical declivity.
- IV.—The distance from the centre of the Red Hills Lake to the point previously described is about 40,820 feet—on a line of country near the present channel, but slightly lower than the level of its bed.
- V.—The average depth of cutting required to lay this piping as above described would be 12 feet.
- VI.—The means I would provide for drawing off the water would be as follows—
 1. An Iron Cylinder 8 or 10 feet in diameter, which would be sunk, say 10 feet below the bottom of the Lake, and extend to about 6 feet above Full Tank level. The total length of this Cylinder would be 42 feet.
 2. The Cylinder would be provided with valves at various depths below the Full Tank level.
 3. The part of the Cylinder below the bottom of the Tank would be excavated and filled with concrete.
 4. The main to Madras would enter through the side of the Cylinder, and be turned up through the concrete to the level of the bottom of the tank.
 5. There would also be a valve on the main inside the Cylinder.
 6. The Cylinder would be approached by a boat from the banks of the tank.
 7. The main must be able to supply at least 6,000,000 gallons per day, but as half of this is drawn off in 6 hours, the main should be large enough to supply 12,000,000 gallons per day, and should be able to do this when the tank is as low as (say) 22'00.
 8. The diameter of such a main would be 44 inches.
 9. At times when the tank is at a higher level than (22'00) the quantity able to be delivered through such a main would be much increased.
 10. The cost of such a scheme as the above is detailed below.

11. I have also shewn in a table what a 44 inch and 36 inch diameter main would supply at varying levels in of the tank, and the cost of a scheme with a 36 inch main.

12. In starting from the tank it would perhaps be the least expensive method to lay the piping along the Channel leading to the Soorapadu Sluic.

13. The iron Cylinder would be floated out by means of pontoons, only so much of it being put together at first as would enable it to be lowered to the bottom of the lake without its higher end being covered.*

14. The following are estimates for the above scheme with a 44 inches and a 36 inch main

	44 inch Main.	36 inch Main.	
Cylinder and Tower in Tank	3,000	3,000	27' dia
Erecting do	2,000	2,000	3000
Machinery in do	2,000	2,000	2000
Cost of Main Lineal yards 13,606 x	7,61,920	5,71,440	444000
Laying do	68,030	37,416	30000
Excavation	36,284	31,750	26000
Cartage of Main	10,714	8,035	7000
Valves in Main	8,400	5,760	5000
Contingencies	890,348	661,401	519000
	89,052	66,149	51000
Total	9,79,400	7,27,550	570900

15. The amount of expenditure in Maintenance would be very little, certainly not more than what it costs to maintain the present Channel.

16. It will be observed that with this scheme there is no filtration, but its advantages over present arrangements would be that water would always be drawn from near the surface of the lake and that it could be drawn off to a depth of 11 feet below the level of present delivery Channel.

17. So long as the water remained above the level of the present Channel, the Head of water in Madras would be at least 2 feet greater than it now is, as there is about 2 feet fall in the length of the Channel.

18. A rise in the level of the water in the Lake above 31'31 does not practically increase the head to any extent in Madras at present, though it increases the quantity of water which can be delivered by the present Channel in a certain time.

19. By means of this scheme the head of water will vary, and when the lake is full the head in Madras will be about 44 feet at the point previously described.

* Such a tower as this composed of iron cylinders might probably be cheaper for the scheme with filtration than a brick tower if the tank stood at a high level.

x Taken as 4.536 pipes each pipe 27'40 lbs @ 80 Rs per Ton
(Say for 27' pipe 1" thick 5560 tons)

TABLE SHewing the quantity of Water which would be delivered in 24 hours by pipes of 44 inches and 36 inches in diameter, at varying levels of the Red Hills Lake.

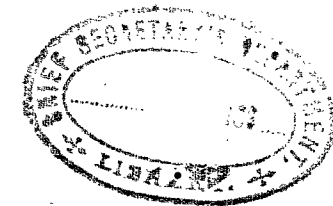
Tank Level.	Head taken as	Delivered by pipes of		
		44 in. diameter.	36 in. diameter.	27" dia
Above mean sea	22.00	1249 12,060,000	861 7,233,000	398 528200
	27.00	1100 13,500,000	309 8,100,000	447 402300
	32.00	1040 14,760,000	284 8,856,000	483 434700
	37.00	1781 16,056,000	1970 9,630,000	526 473400
	42.00	1930 17,100,000	1140 10,260,000	559 503100
	46.00	2023 18,198,000	1213 10,917,000	596 536400

NOTE.—The smaller quantities underlined thus — are cubic feet per minute.

The larger quantities are gallons per day.

In reading the above table it should be remembered that Madras requires 6,000,000 gallons per day or 666 cubic feet per minute, but that during certain hours the consumption is double this quantity.

J. A. JONES.



APPENDIX IV.

RED HILLS TANK.

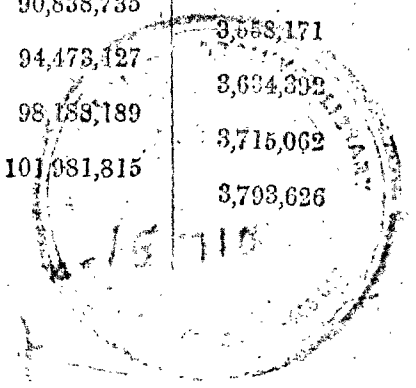
Table showing Cubical Contents at half foot vertical intervals.

28

Height above mean sea level.	Contents at level in column 1.	Difference of cubical contents at each half foot.	Height above mean sea level.	Contents at level in column 1.	Difference of cubical contents at each half foot.
1	2	3	1	2	3
FEET.	CUBIC YARDS.	CUBIC YARDS.	FEET.	CUBIC YARDS.	CUBIC YARDS.
22-31	944,514		34-31	33,646,893	2,103,979
22-81	1,485,647	541,133	35-31	35,826,419	2,179,526
23-31	2,099,961	614,314	35-81	38,080,312	2,253,893
23-81	2,787,152	687,191	36-31	40,406,379	2,326,067
24-31	3,546,648	759,496	36-81	42,809,544	2,403,165
24-81	4,379,655	833,007	37-31	45,318,050	2,509,506
25-31	5,285,442	905,787	37-81	47,899,712	2,581,662
25-81	6,248,189	962,747	38-31	50,544,433	2,644,721
26-31	7,266,081	1,017,892	38-81	53,248,891	2,704,458
26-81	8,339,998	1,073,917	39-31	56,013,395	2,764,504
27-31	9,467,808	1,127,810	39-81	58,840,739	2,827,844
27-81	10,651,052	1,183,244	40-31	61,767,999	2,927,260
28-31	11,889,957	1,238,905	40-81	64,667,822	2,900,823
28-81	13,141,584	1,251,627	41-31	67,651,247	2,983,425
29-31	14,475,912	1,334,328	41-81	70,716,557	3,065,310
29-81	15,874,637	1,397,725	42-31	73,861,778	3,145,221
30-31	17,338,013	1,464,376	42-81	77,090,852	3,229,074
30-81	18,873,401	1,535,388	43-31	80,401,616	3,310,764
31-31	20,477,034	1,603,633	43-81	83,800,897	3,399,231
31-81	22,132,523	1,655,489	44-31	87,280,564	3,479,667
32-31	23,864,963	1,732,440	44-81	90,838,735	3,558,171
32-81	25,671,308	1,806,345	45-31	94,473,127	3,634,392
33-31	27,552,497	1,881,189	45-81	98,183,189	3,715,062
33-81	29,510,484	1,957,987	46-31	101,981,815	3,793,626
34-31	31,542,914	2,032,430			

2

28 5 24 11 17
1827



செப்பனிருப்பவர் பெயர் D.உரதன்
(Re-Winding Work)
செப்பனிருப்பதற்காக, எடுத்துக்
கொள்ளப்பட்ட நாள். 18.11.91
செப்பனுக்கு முடித்த நாள் 28.11.91
சுப்பார்க்கும் ஆராய்ச்சி
மு.பா.எ. உதவியாளர்
கையொப்பம். கையொப்பம்.